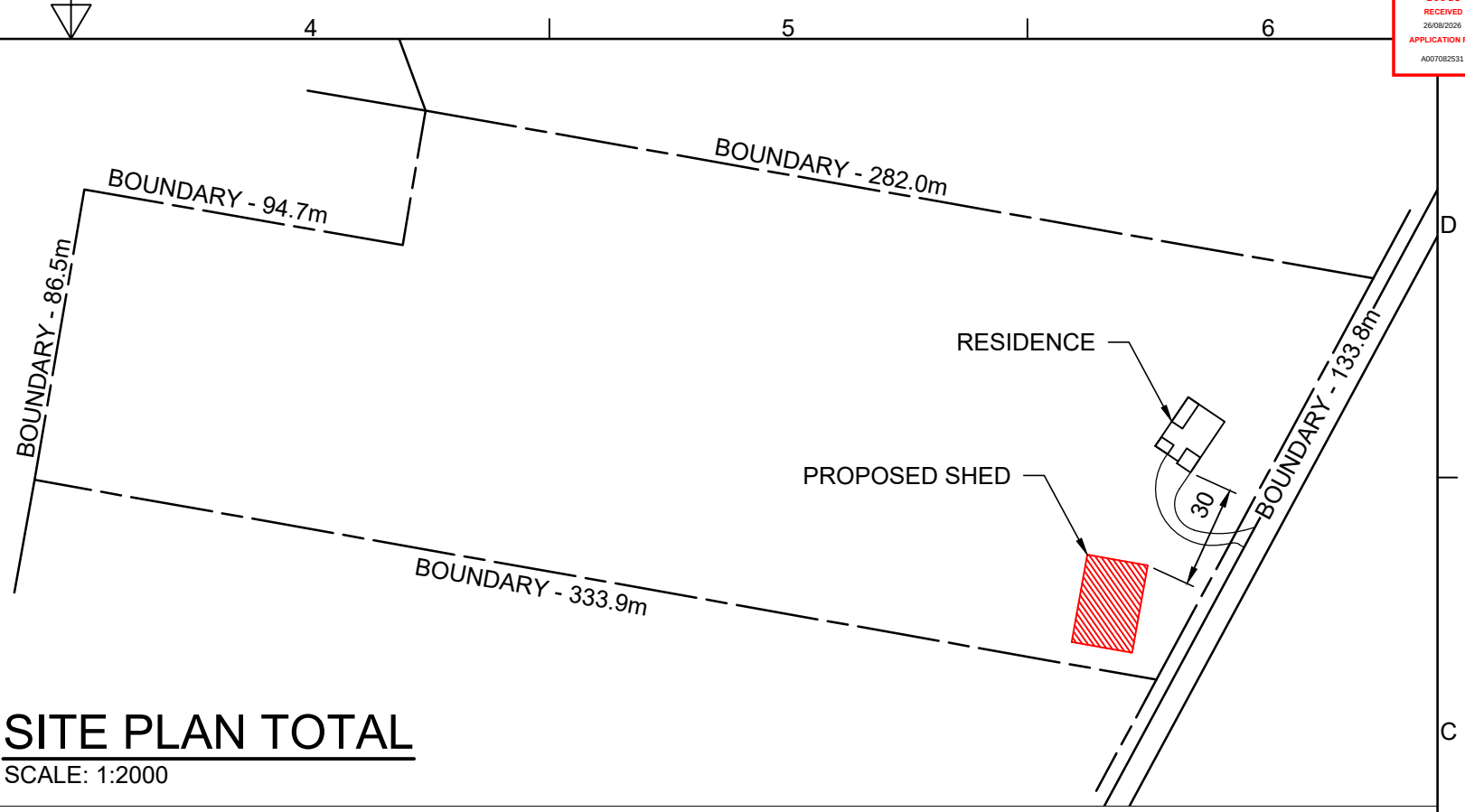
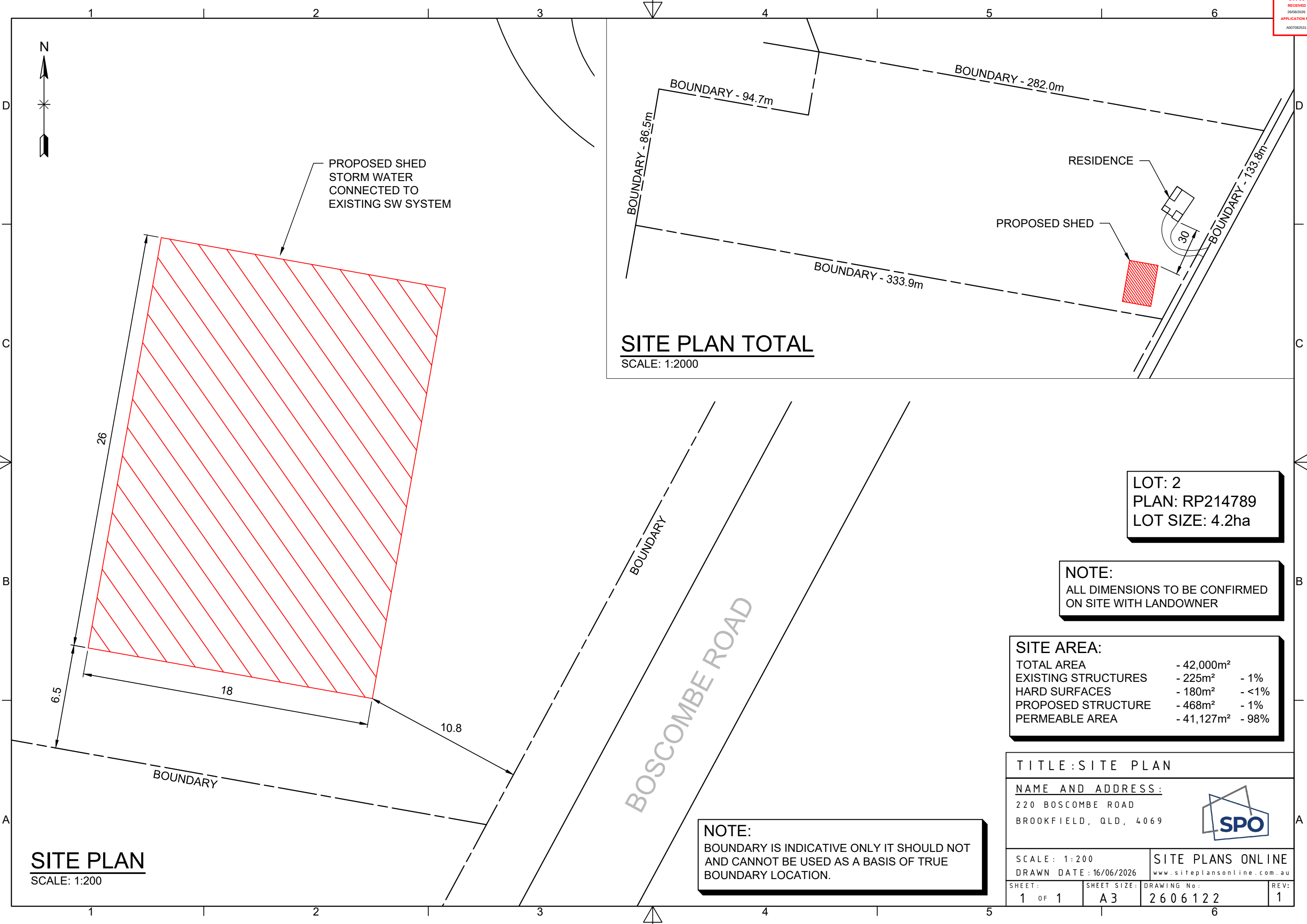




LOT: 2
PLAN: RP214789
LOT SIZE: 4.2ha

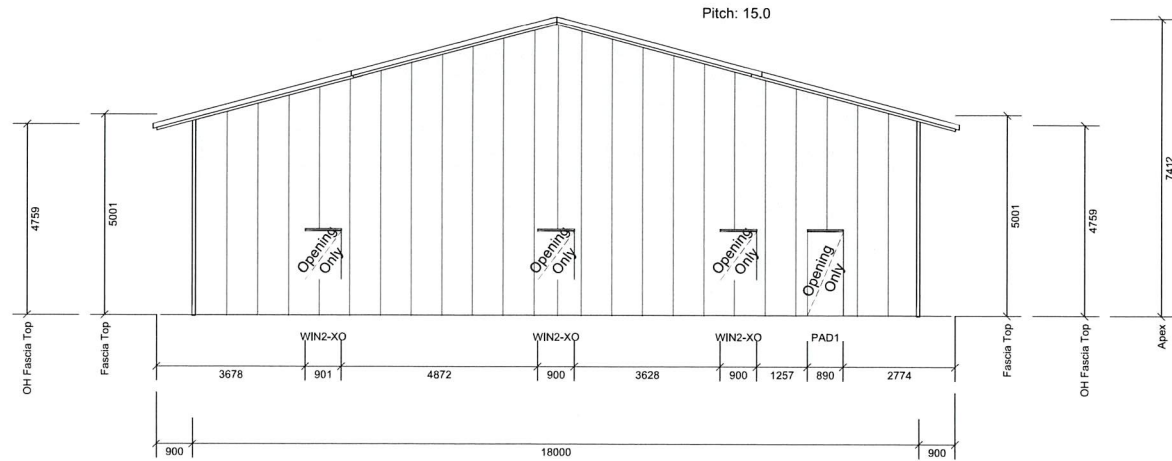
NOTE:
ALL DIMENSIONS TO BE CONFIRMED
ON SITE WITH LANDOWNER

SITE AREA:		
TOTAL AREA	- 42,000m ²	
EXISTING STRUCTURES	- 225m ²	- 1%
HARD SURFACES	- 180m ²	- <1%
PROPOSED STRUCTURE	- 468m ²	- 1%
PERMEABLE AREA	- 41,127m ²	- 98%

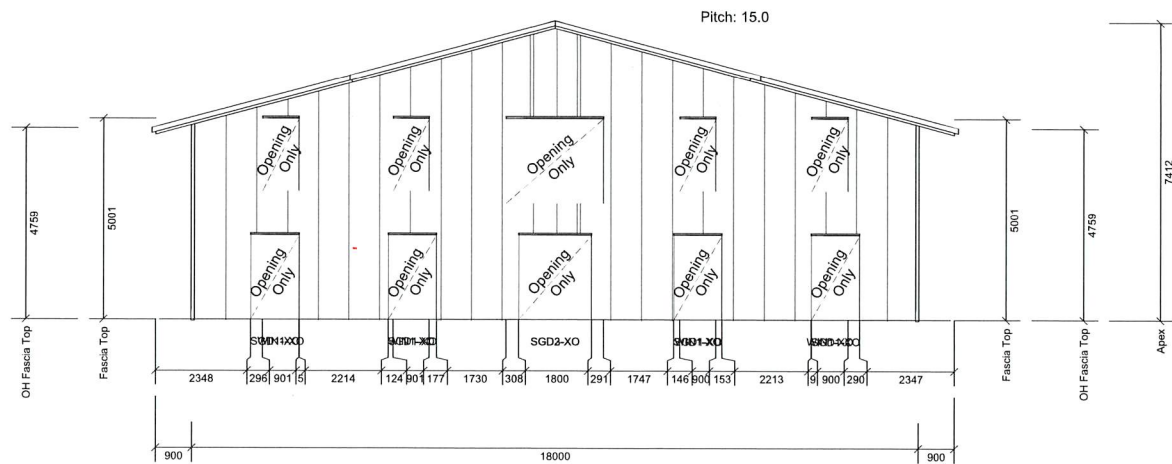
TITLE : SITE PLAN			
NAME AND ADDRESS : 220 BOSCOMBE ROAD BROOKFIELD, QLD, 4069			
SCALE : 1:200 DRAWN DATE : 16/06/2026		SITE PLANS ONLINE www.siteplansonline.com.au	
SHEET : 1 OF 1	SHEET SIZE : A3	DRAWING No : 2606122	REV : 1

NOTE:
BOUNDARY IS INDICATIVE ONLY IT SHOULD NOT
AND CANNOT BE USED AS A BASIS OF TRUE
BOUNDARY LOCATION.





FRONT ELEVATION



BACK ELEVATION



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ABN: 84 849 102 256

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QBCC Lic. 15249426

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E brisbane.west@rfsteelbuildings.com.au

PROJECT NO: **P20816Q4**

CUSTOMER: Jack Dore

PROJECT NAME: **Jack Dore**

JOB NAME: **Jack Dore Rev #3**

SITE: 220 Boscombe Rd
Brookfield, QLD 4069

LOT: 2 RP/SP: 214 789

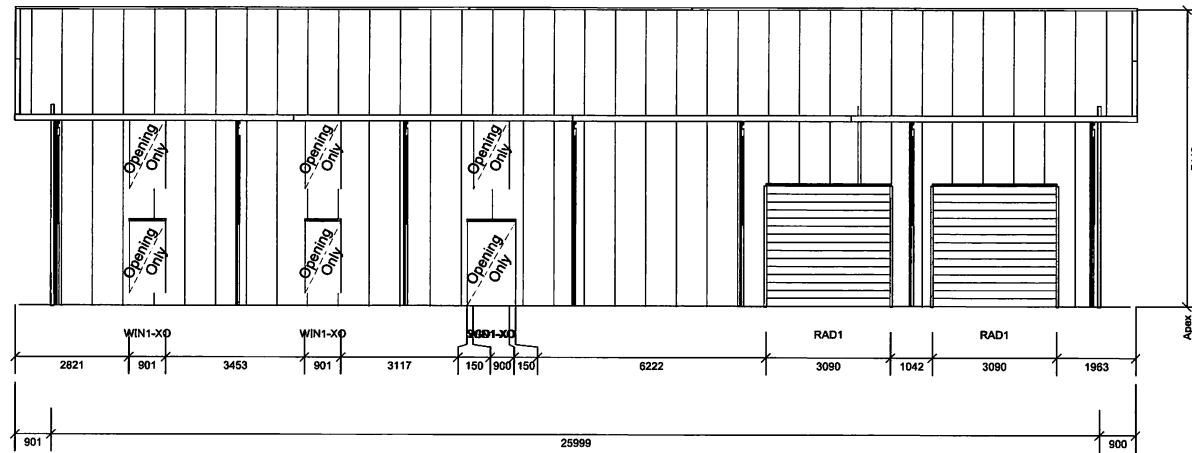
DRAWING No: **J5682-Dore:Elevation**

DATE: **10/06/2026**

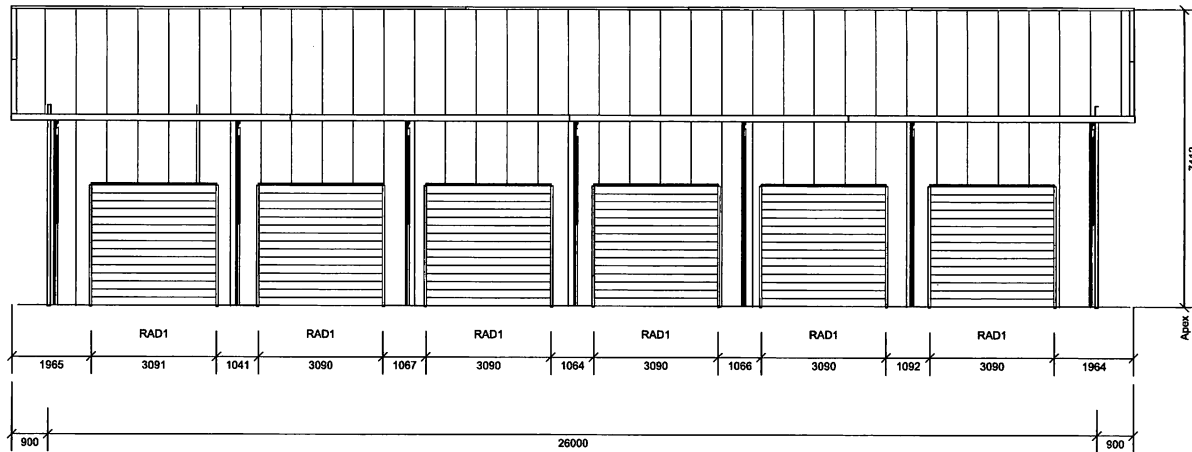
ULT WIND SPEED: **60.54 m/s**

SERVICEABILITY: **41.42 m/s**

George Zuev 12 JUN 2026
RPEQ 7551



LEFT ELEVATION



RIGHT ELEVATION



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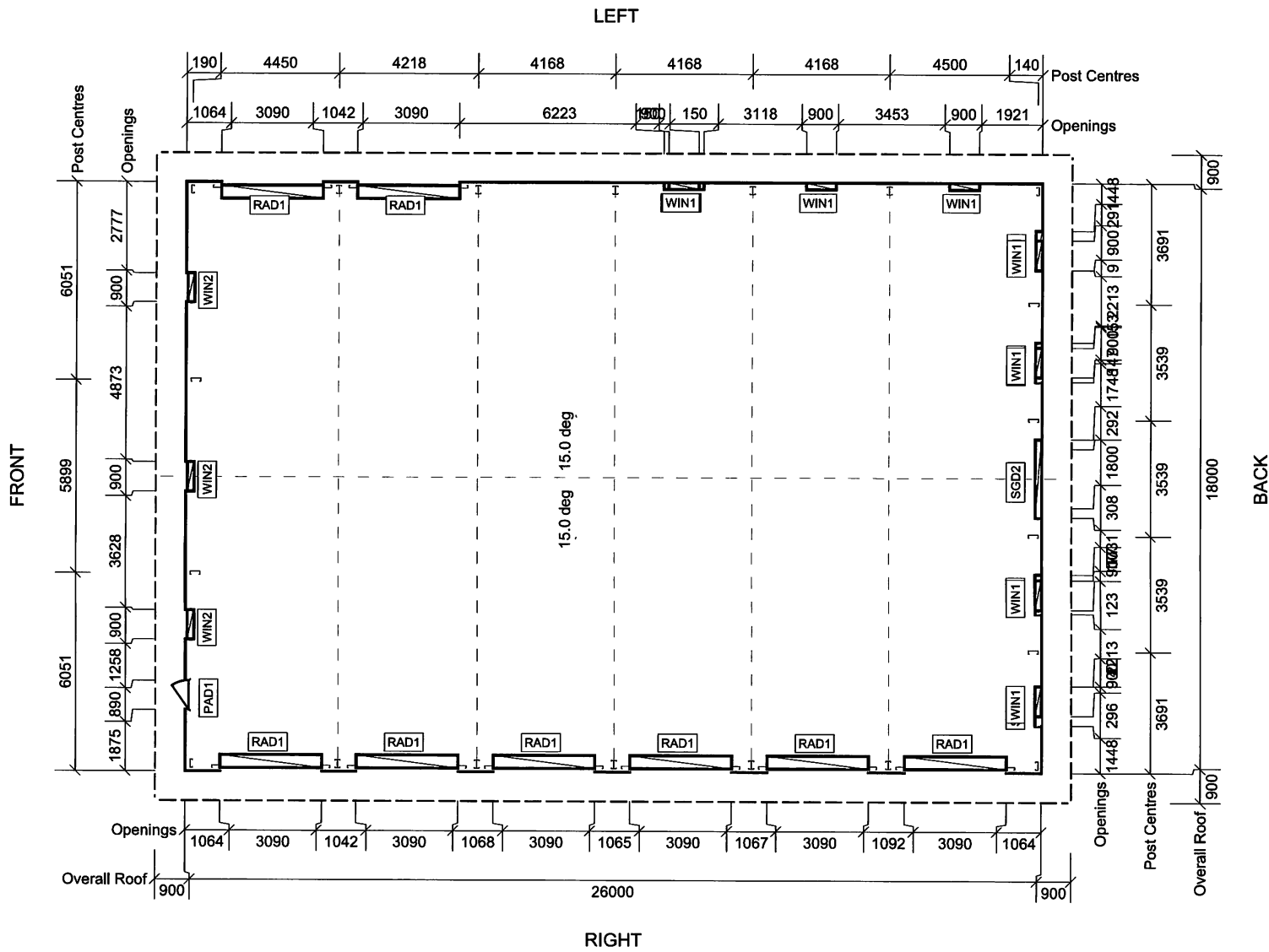
LOT: 2 RP/SP: 214 789

DRAWING No: **J5682-Dore:Elevation**

DATE: **10/06/2026**

ULT WIND SPEED: **60.54 m/s**
SERVICEABILITY: **41.42 m/s**

George Zuev 12 JUN 2026
RPEQ 7551

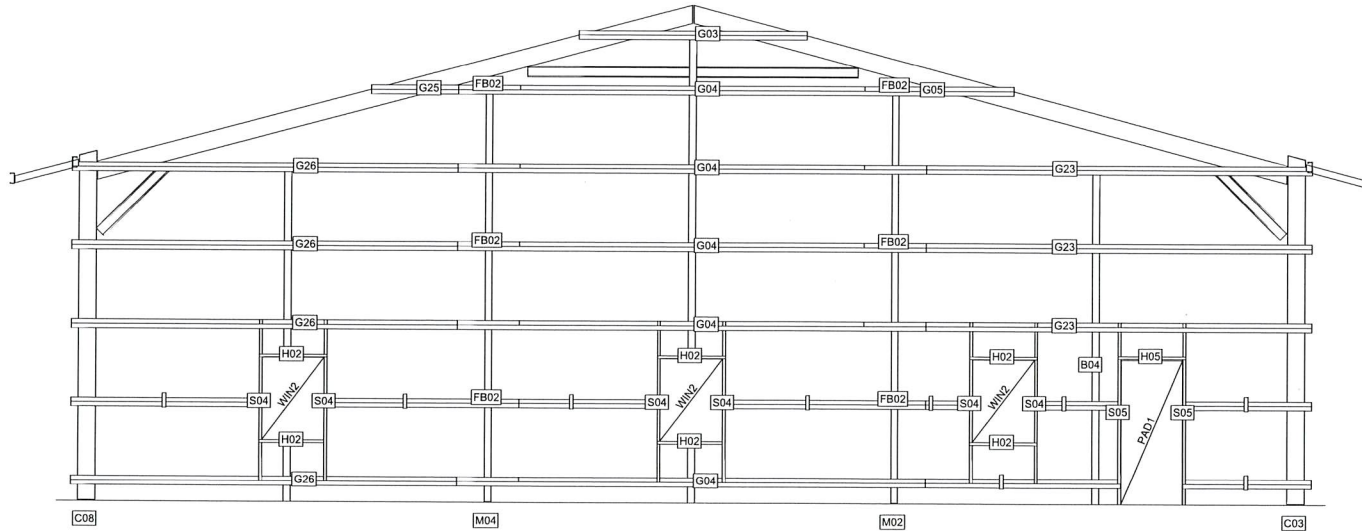


Opening Legend	
PAD1-OP	2100h x 890w
RAD1	3000h x 3090w
SGD1-OP	2100h x 1200w
SGD2-OP	2100h x 2400w
SGD3-OP	2100h x 1800w
WIN1-OP	1800h x 900w
WIN2-OP	1200h x 900w

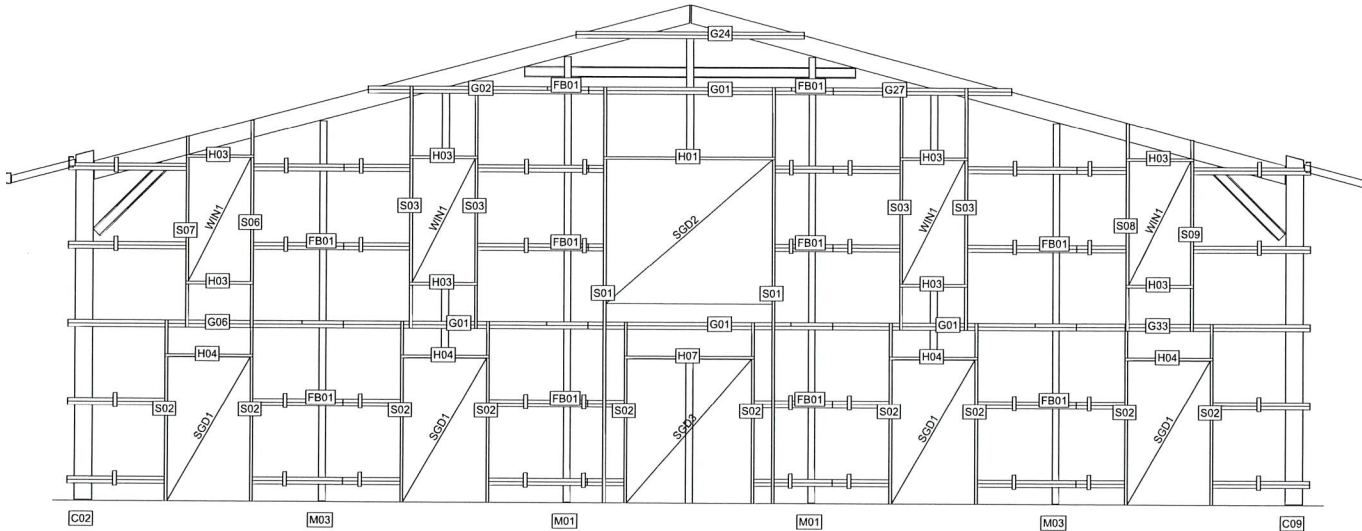
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RPEQ 7551
12 JUN 2026

Floor Plan

 R&F Steel Buildings Brisbane West ABN: 84 849 102 256 R&F Steel Buildings Brisbane West QBCC Lic. 15249426 12 / 186 Douglas St Oxley QLD 4075 T 0432 589 753 E brisbane.west@rfsteelbuildings.com.au	PROJECT NO: P20816Q4	CUSTOMER: Jack Dore	SITE: 220 Boscombe Rd Brookfield, QLD 4069	DATE: 10/06/2026
	PROJECT NAME: Jack Dore			LOT: 2 RP/SP: 214 789
	JOB NAME: Jack Dore Rev #3	DRAWING No: J5682-Dore:Floor Plan		

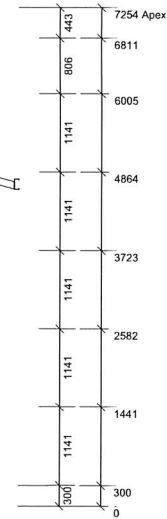
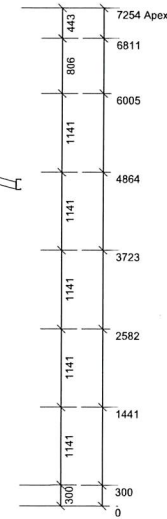


Front



Back

Wall Girt Layout - End Elevations



George Zuev
RPEQ 7551
12 JUN 2026

Label	Size	Qty	Length
B01	PC10012	2	6056
B03	PC10012	1	6862
B04	PC10012	2	4928
B07	PC10012	2	6875
C02	PC25024	1	5085
C03	PC25024	1	5085
C08	PC25024	1	5085
C09	PC25024	1	5085
FB01	RNFFBC30051	10	51
FB02	RNFFBC30063	6	64
G01	PZ10012	16	4139
G02	PZ10012	1	3189
G03	PZ15012	1	3303
G04	PZ15012	6	6798
G05	PZ15012	1	2160
G06	PZ10012	5	3991
G23	PZ15012	5	6500
G24	PZ10012	1	3303
G25	PZ15012	1	2160
G26	PZ15012	5	6500
G27	PZ10012	1	3189
G33	PZ10012	5	3991
H01	RNF10F129GA	1	2480
H02	RNF10F130GA	6	980
H03	RNF10F129GA	8	980
H04	RNF10F129GA	4	1280
H05	RNF10F130GA	1	985
H07	RNF10F129GA	1	1880
M01	PC30024	2	6486
M02	PC30024	1	6169
M03	PC30024	2	5537
M04	PC30024	1	6169
S01	RNF10F129GA	2	6056
S02	RNF10F129GA	10	2633
S03	RNF10F129GA	4	3525
S04	RNF10F130GA	6	2410
S05	RNF10F130GA	2	2646
S06	RNF10F129GA	1	3019
S07	RNF10F129GA	1	2778
S08	RNF10F129GA	1	3018
S09	RNF10F129GA	1	2777
WIN1	PC10012	110	900
WIN2	PC10012	110	900

Opening Legend			
PAD1-OP	2100h x 890w		
SGD1-OP	2100h x 1200w		
SGD2-OP	2100h x 2400w		
SGD3-OP	2100h x 1800w		
WIN1-OP	1800h x 900w		
WIN2-OP	1200h x 900w		
Achieved Bracing		Required Bracing	
163.998383 kN		122.346298 kN	

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PROJECT NO: **P20816Q4**

CUSTOMER: Jack Dore

PROJECT NAME: **Jack Dore**

JOB NAME: **Jack Dore Rev #3**

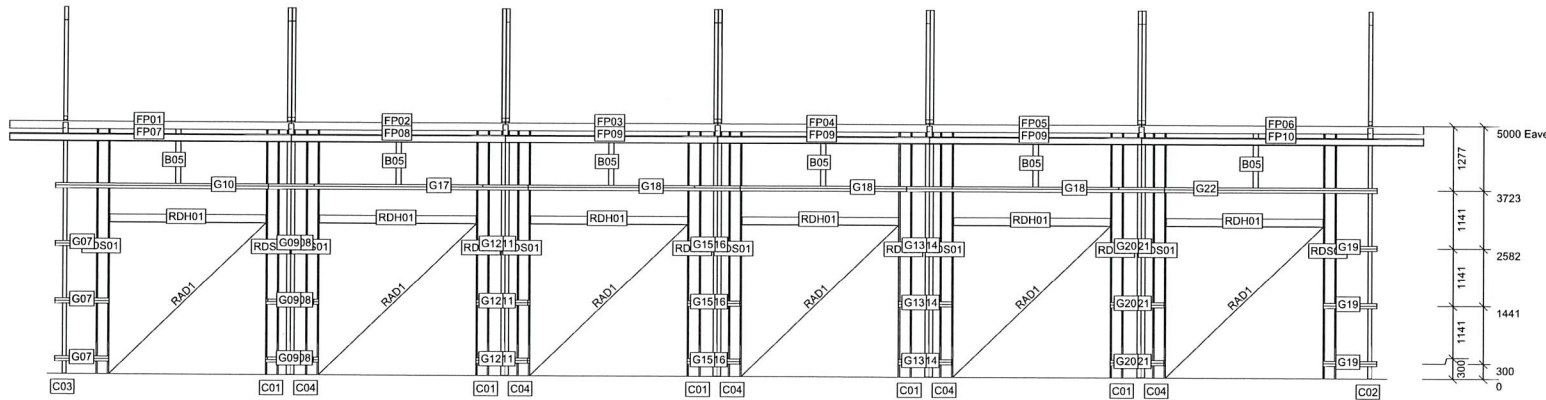
SITE: 220 Boscombe Rd
Brookfield, QLD 4069

LOT: 2 RP/SP: 214 789

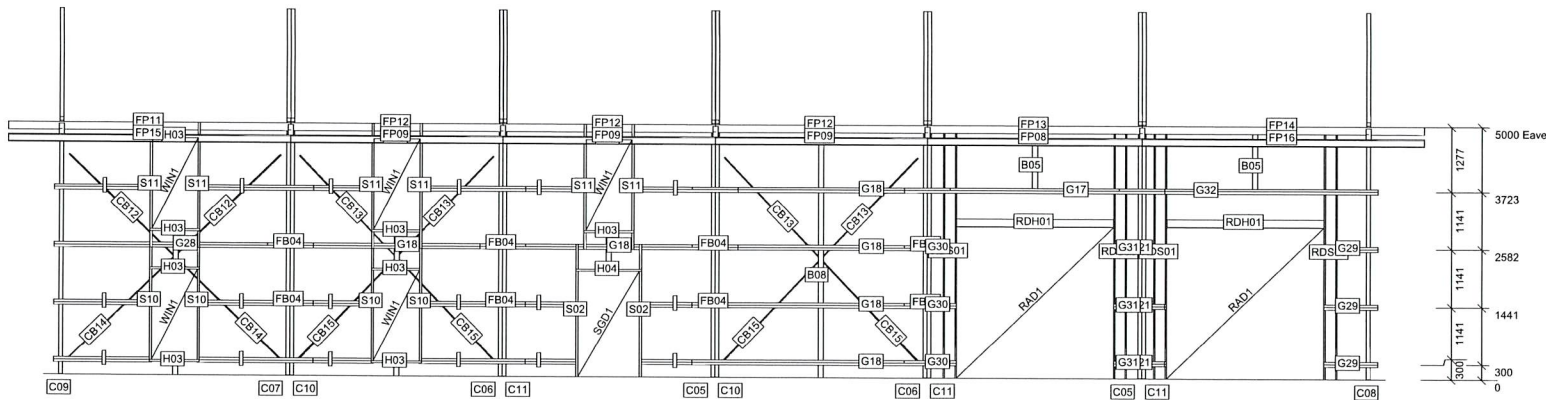
DRAWING No: **J5682-Dore:Wall Girt Layout**

DATE: **10/06/2026**

ULT WIND SPEED: **60.54 m/s**
SERVICEABILITY: **41.42 m/s**



Right



Left

Wall Girt Layout - Side Elevations

George Zuev
RPEQ 7551
12 JUN 2026

Label	Size	Qty	Length
B05	PC10012	9	1328
B08	PC10012	5	5000
C01	PC25024	5	5085
C02	PC25024	1	5085
C03	PC25024	1	5085
C04	PC25024	5	5085
C05	PC25024	2	5085
C06	PC25024	2	5085
C07	PC25024	1	5085
C08	PC25024	1	5085
C09	PC25024	1	5085
C10	PC25024	2	5085
C11	PC25024	3	5085
CB12	101M46G-3020-PI	2	2892
CB13	101M46G-3020-PI	4	2762
CB14	101M46G-3020-PI	2	2892
CB15	101M46G-3020-PI	4	2762
FB04	RNFBC250D51	8	51
FP01	PC15015	1	5548
FP02	PC15015	1	4217
FP03	PC15015	1	4167
FP04	PC15015	1	4167
FP05	PC15015	1	4167
FP06	PC15015	1	5548
FP07	PC15015	1	5548
FP08	PC15015	2	4217
FP09	PC15015	6	4167
FP10	PC15015	1	5548
FP11	PC15015	1	5547
FP12	PC15015	3	4167
FP13	PC15015	1	4217
FP14	PC15015	1	5547
FP15	PC15015	1	5547
FP16	PC15015	1	5547
G07	PZ10012	3	1064
G08	PZ10012	3	1005
G09	PZ10012	3	936
G10	PZ10012	1	5090
G11	PZ10012	3	945
G12	PZ10012	3	1022
G13	PZ10012	3	1045
G14	PZ10012	3	921
G15	PZ10012	3	1032
G16	PZ10012	3	932
G17	PZ10012	2	5117
G18	PZ10012	15	5067
G19	PZ10012	3	1064
G20	PZ10012	3	1056
G21	PZ10012	6	936
G22	PZ10012	1	5090
Bracing Legend			
RAD1	G28	PZ10012	3000h x 3090w
SGD1	OP	PZ10012	2100h x 1200w
WIN1	OP	PZ10012	1800h x 900w
G31	PZ10012	3	1066
G32	PZ10012	3	1066
H03	RNF10F129GA	10	950
H04	RNF10F129GA	10	950
RDH01	PC15015	8	3080
RDS01	PC25024	16	5990
S02	RNF10F129GA	2	2633
S10	RNF10F129GA	2	2333
S11	RNF10F129GA	2	2469
PC10012	SERVICEABILITY:	46	300

Achieved Bracing Required Bracing
163.998kN 122.346kN

R&F STEEL BUILDINGS
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ABN: 84 849 102 256

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PROJECT NO: **P20816Q4**

CUSTOMER: Jack Dore

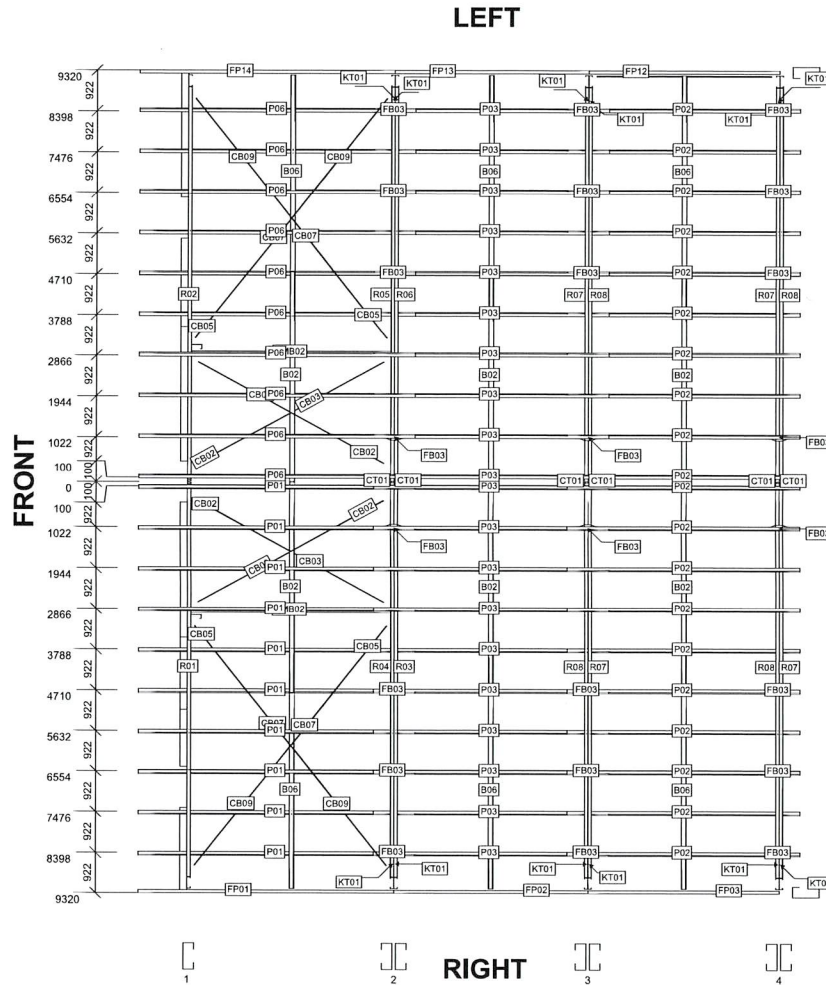
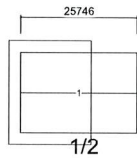
PROJECT NAME: **Jack Dore**

JOB NAME: **Jack Dore Rev #3**

SITE: 220 Boscombe Rd
Brookfield, QLD 4069

LOT: 2 RP/SP: 214 789

DRAWING No: **J5682-Dore:Wall Girt Layout**



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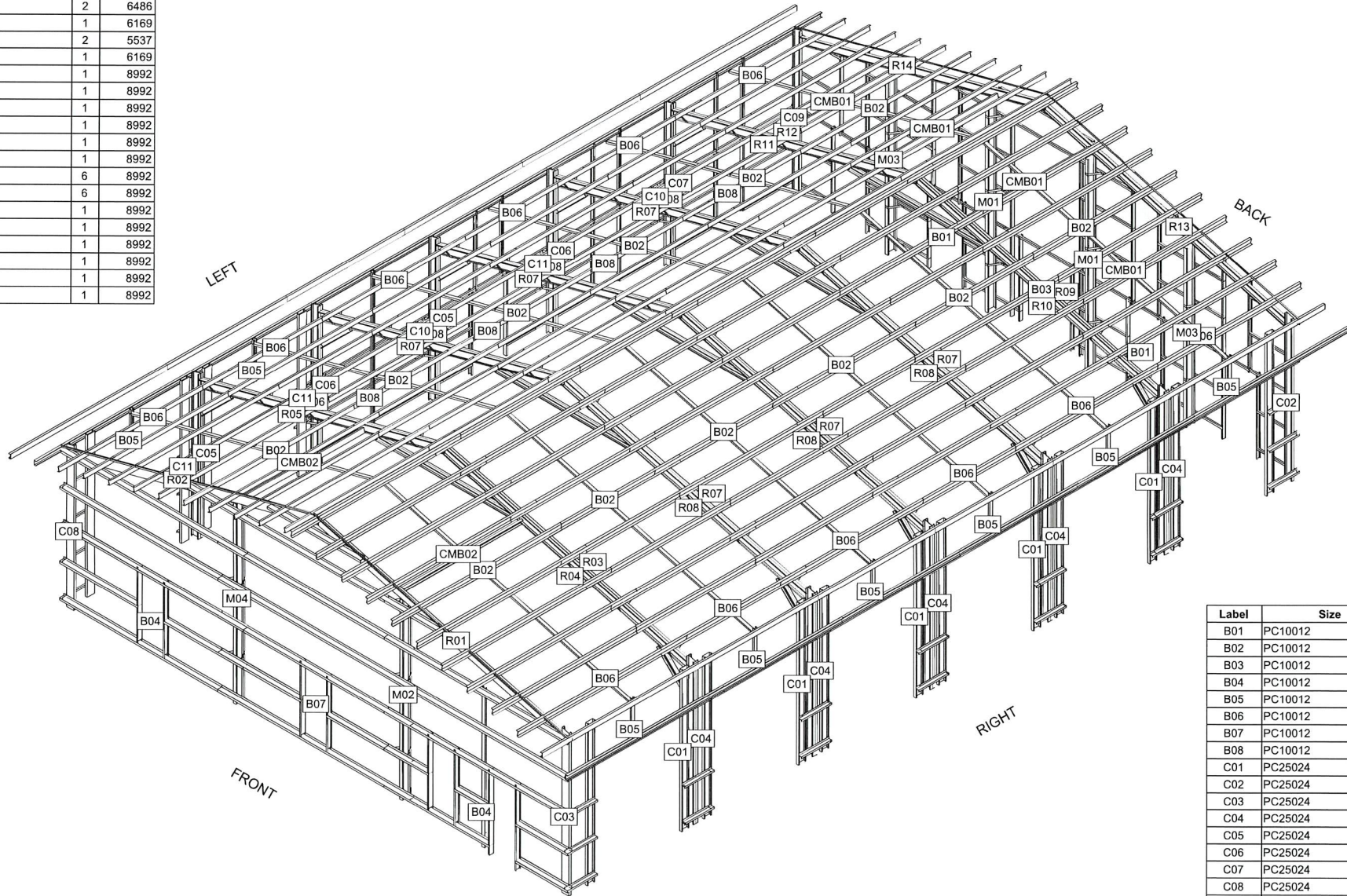
Label	Size	Qty	Length
B02	PC10012	7	4757
B06	PC10012	7	4554
CB02	101M46G-3012-PI	4	2311
CB03	101M46G-3012-PI	4	2311
CB05	101M46G-3012-PI	4	2276
CB07	101M46G-3012-PI	4	2276
CB09	101M46G-3012-PI	4	2276
CMB02	PC15012	2	4336
CT01	PC15015	6	4797
FB03	RNFFBC250D63	24	64
FP01	PC15015	1	5548
FP02	PC15015	1	4217
FP03	PC15015	1	4167
FP12	PC15015	1	4167
FP13	PC15015	1	4217
FP14	PC15015	1	5547
KT01	PC15015	12	1371
P01	PZ15012	10	5998
P02	PZ15012	20	5067
P03	PZ15012	20	5117
P06	PZ15012	10	5998
R01	PC25024	1	8992
R02	PC25024	1	8992
R03	PC25024	1	8992
R04	PC25024	1	8992
R05	PC25024	1	8992
R06	PC25024	1	8992
R07	PC25024	4	8992
R08	PC25024	4	8992

Achieved Bracing	Required Bracing
217.769302 kN	46.986080 kN

Roof Purlin Plan (Env: 1) - 1 of 2

 R&F Steel Buildings Brisbane West ABN: 84 849 102 256	PROJECT NO: P20816Q4 PROJECT NAME: Jack Dore JOB NAME: Jack Dore Rev #3	CUSTOMER: Jack Dore LOT: 2 DRAWING No: J5682-Dore:Roof Purlin Plan	SITE: 220 Boscombe Rd Brookfield, QLD 4069 RP/SP: 214 789	DATE: 10/06/2026
				ULT WIND SPEED: 60.54 m/s SERVICEABILITY: 41.42 m/s

Label	Size	Qty	Length
CMB02	PC15012	2	4336
M01	PC30024	2	6486
M02	PC30024	1	6169
M03	PC30024	2	5537
M04	PC30024	1	6169
R01	PC25024	1	8992
R02	PC25024	1	8992
R03	PC25024	1	8992
R04	PC25024	1	8992
R05	PC25024	1	8992
R06	PC25024	1	8992
R07	PC25024	6	8992
R08	PC25024	6	8992
R09	PC25024	1	8992
R10	PC25024	1	8992
R11	PC25024	1	8992
R12	PC25024	1	8992
R13	PC25024	1	8992
R14	PC25024	1	8992

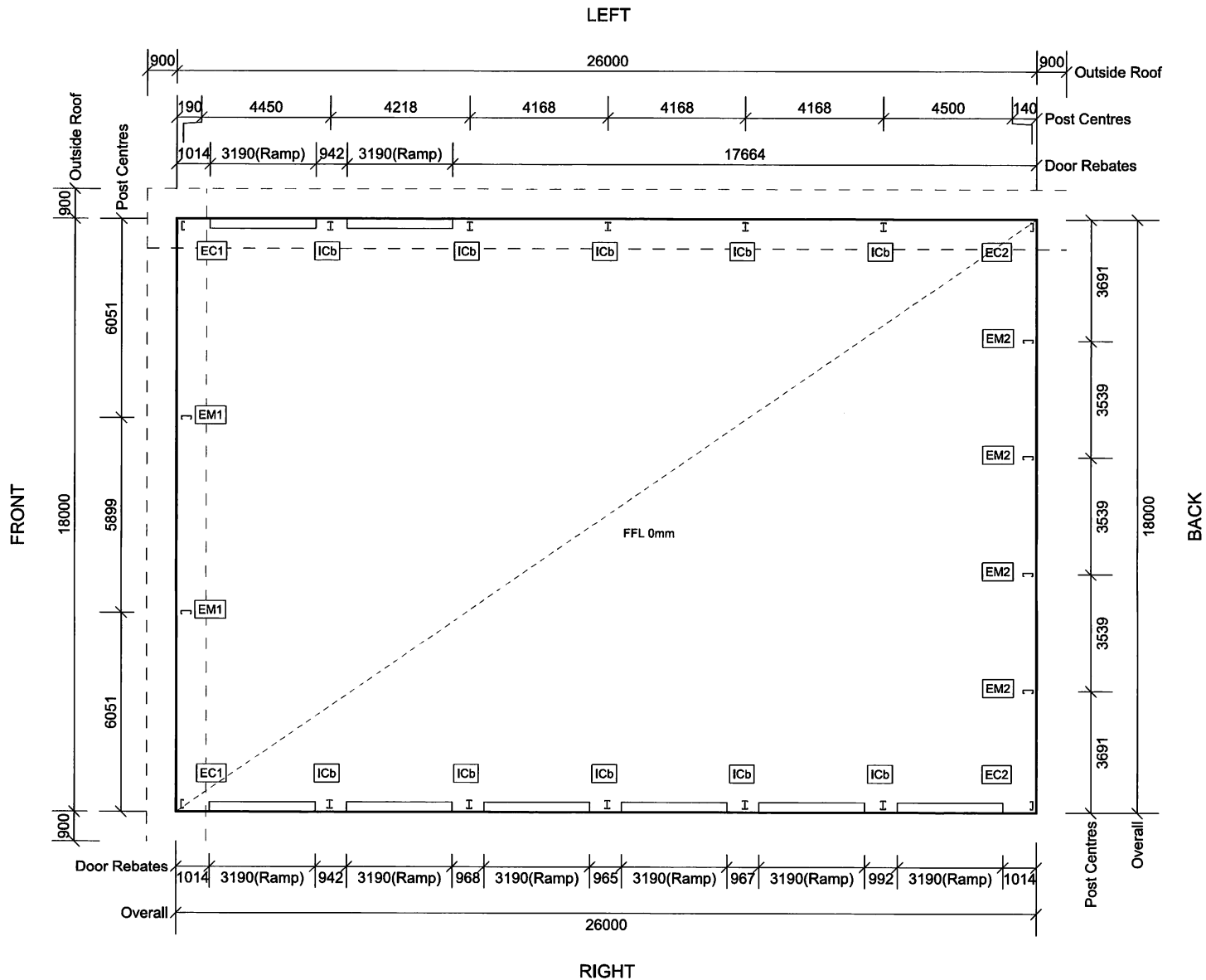


Label	Size	Qty	Length
B01	PC10012	2	6056
B02	PC10012	14	4757
B03	PC10012	1	6862
B04	PC10012	2	4928
B05	PC10012	9	1328
B06	PC10012	14	4554
B07	PC10012	2	6875
B08	PC10012	5	5000
C01	PC25024	5	5085
C02	PC25024	1	5085
C03	PC25024	1	5085
C04	PC25024	5	5085
C05	PC25024	2	5085
C06	PC25024	2	5085
C07	PC25024	1	5085
C08	PC25024	1	5085
C09	PC25024	1	5085
C10	PC25024	2	5085
C11	PC25024	3	5085
CMB01	PC15012	4	4386

ISO Front Right

 R&F Steel Buildings Brisbane West ABN: 84 849 102 256	PROJECT NO: P20816Q4	CUSTOMER: Jack Dore	SITE: 220 Boscombe Rd Brookfield, QLD 4069	DATE: 10/06/2026
	PROJECT NAME: Jack Dore		LOT: 2 RP/SP: 214 789	ULT WIND SPEED: 60.54 m/s
	JOB NAME: Jack Dore Rev #3		DRAWING No: J5682-Dore:ISO Plan	SERVICEABILITY: 41.42 m/s

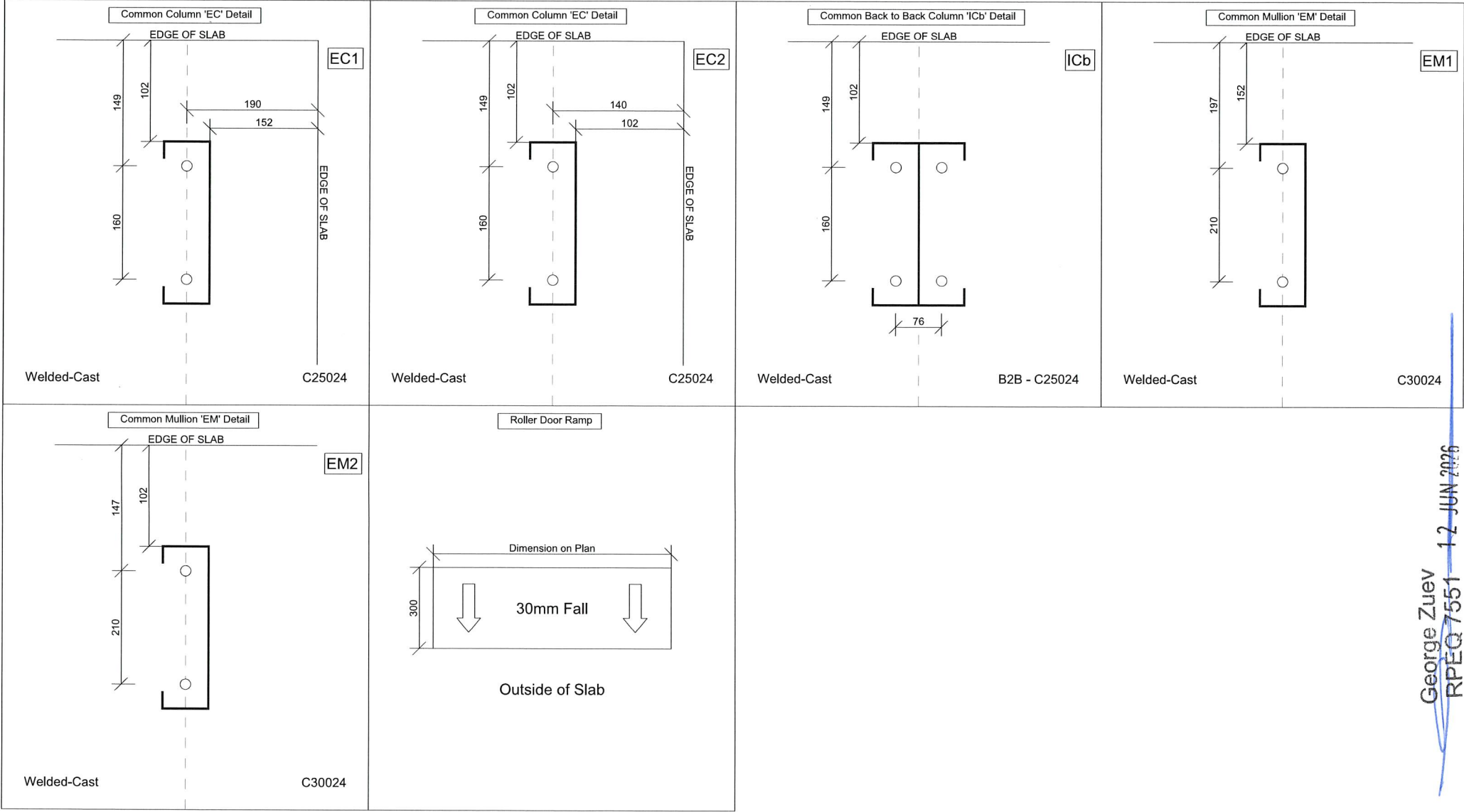
George Zuev 12 JUN 2026
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Slab / Footing Plan

 R&F Steel Buildings Brisbane West ABN: 84 849 102 256	R&F Steel Buildings Brisbane West QBCC Lic. 15249426	PROJECT NO: P20816Q4	CUSTOMER: Jack Dore	SITE: 220 Boscombe Rd Brookfield, QLD 4069	DATE: 10/06/2026
	12 / 186 Douglas St Oxley QLD 4075 T 0432 589 753 E brisbane.west@rfsteelbuildings.com.au	PROJECT NAME: Jack Dore		LOT: 2 RP/SP: 214 789	ULT WIND SPEED: 60.54 m/s SERVICEABILITY: 41.42 m/s
		JOB NAME: Jack Dore Rev #3		DRAWING No: J5682-Dore:Slab/Footing Plan	

George Zuev
RPEQ 7551
12 JUN 2026



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RPEQ 7551
12 JUN 2026

Post Details

 R&F Steel Buildings Brisbane West ABN: 84 849 102 256	PROJECT NO: P20816Q4		CUSTOMER: Jack Dore		SITE: 220 Boscombe Rd Brookfield, QLD 4069		DATE: 10/06/2026	
	PROJECT NAME: Jack Dore				LOT: 2		RP/SP: 214 789	
	JOB NAME: Jack Dore Rev #3				DRAWING No: J5682-Dore:Slab/Footing Plan		ULT WIND SPEED: 60.54 m/s SERVICEABILITY: 41.42 m/s	

WIND SPEED CLASSIFICATIONS

REGION	PERM. STRESS WIND CLASS.	AS4055. WIND CAT.	Vp.	Vu.	R&F WIND CAT.	Cpi.	OPENINGS
A&B	W33	N2	33	40	N40	0.2,-0.3	NO DOMINANT
A&B	W33	N2	33	40	N40D	0.7,-0.65	DOMINANT
A&B	W41N	N3	41	50	N50	0.2,-0.3	NO DOMINANT
A&B	W41N	N3	41	50	N50D	0.7,-0.65	DOMINANT
A&B	W50N	N4	50	61	N61	0.2,-0.3	NO DOMINANT
A&B	W50N	N4	50	61	N61D	0.7,-0.65	DOMINANT
C&D	W41C	C1	41	50	C50	0.7,-0.65	
C&D	W50C	C2	50	61	C61	0.7,-0.65	
C&D	W60C	C3	60	73	C73	0.7,-0.65	
C&D	W70C	C4	70	85	C85	0.7,-0.65	

GENERAL NOTES

- During construction, the structure shall be maintained in a stable condition. Construction loads must not exceed the capacity of the structure at the time of loading.
- All workmanship & materials shall be in accordance with the relevant current SANS standards & codes of practice except where varied by the contract documents or of the by-laws of the local authority.
- Wind loads have been assessed in accordance with AS/NZ1170.2
- Live loading are in accordance with AS/NZ1170.1
- Enclosed Building' Member Schedule denotes -0.3,+0.2kPa internal pressure coefficient used. 'Dominant Opening' Member Schedule denotes -0.65,+0.7kPa internal pressure coefficient used.
- Certification of Design is subject to a Site Specific Design Certificate being issued by an engineer for the structure in question.
- This plan is suitable for use as Building Class 6, 7, 8, 9a, 9b & 10a. Refer to Engineer for Class 1 (habitable dwelling) design or if internal lining is being fitted.
- This plan is suitable for Building Importance Levels 1, 2, 3 & 4
- This plan is suitable for Terrain categories 2, 2.5, 3 & 4. Refer to Engineer for Terrain Cat 1.
- Additional awning, carports, sheds & skillion buildings may be attached to these structure by using any standard Answer Software Engineering Plan.

SNOW LOADING - (Kpa)

NBR WIND CAT.	ROOF PITCH			
	10°	15°	20°	25°
N40 / N40D	1.0	1.5	2.0	2.0
N50 / N50D	1.3	1.8	2.4	3.5
N61 / C50	1.5	2.2	3.2	4.0
N61D / C61	1.8	2.6	4.0	5.0

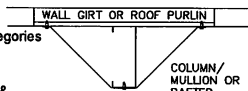
These standard drawings are suitable for the following loads. (Refer AS1170.3)

COMPLETE INUNDATION OF FLOOD WATER

NBR WIND CAT.	Mtr. Per second
N50 / N50D	1.0mtr / sec.
N61 / C50	1.25mtr / sec.
N61D / C61	1.5mtr / sec.

KNEE BRACE SELECTION TABLE

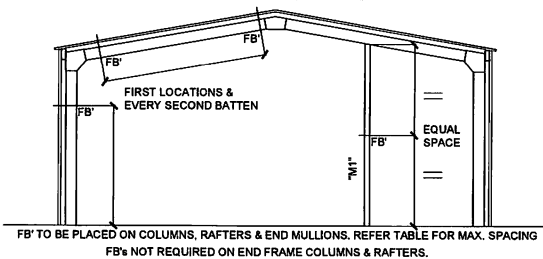
SECTION	KNEE BRACE SIZE	END PLATE	CONNECTION
C200	50x3 SHS	8mm	2x M16 GR 8.8
C250	65x3 SHS	10mm	2x M16 GR 8.8
C300	75x3 SHS	12mm	2x M16 GR 8.8
2xC200's	65x3 SHS	10mm	4x M16 GR 8.8
2xC250's	75x3 SHS	10mm	4x M16 GR 8.8
2xC300's	75x3 SHS	12mm	4x M16 GR 8.8



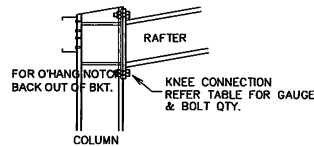
FLY BRACE CONNECTION DETAIL

FIX 0.4 FLY BRACE WITH 2/12x20 TEKS PER FIXING POINT FOR TOP HAT PURLIN/GIRT CONNECTIONS. FIX 30x1.0 FLY BRACE WITH 2/12x20 TEKS PER FIXING POINT FOR C/Z PURLIN/GIRT CONNECTIONS.

ONE SIDE ONLY REQD. AT RIDGE



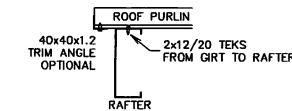
FLY BRACE LOCATIONS



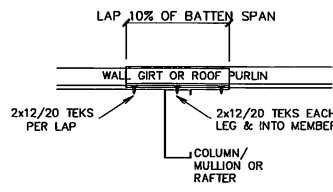
KNEE DETAIL

BRACING NOTES

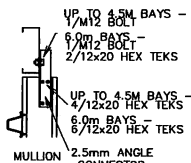
- Long wall cross bracing requirements are provided on the Building Structure Compliance Statement. Ensure sufficient strap or rod braces are provided using the Bracing Capacities table.
- Roof cross bracing shall be placed in any bay, but must be placed in the same bay. Wall cross bracing shall be placed in either the same bay or the adjacent bay as roof bracing. Wall cross bracing may be placed on one side of structure if bay/s is unavailable due to opening/s.
- Strap Bracing tie-downs to be 3/12x20 teks for 30mm & 32mm strap & 6/12x20 teks for 38mm & 50mm strap. For C85 Wind Cat's, use 14x25 teks.
- Doubling up strap cross bracing in a single bay is allowed, but should only be done where alternative bays are not available to brace. Add 90% of bracing value for second set of strap cross brace.
- End wall cross bracing is required on all End Frames. Building up to 1.5 x longer than span - use 50x12mm strap, up to 2 x longer than span - use 12mm rod & up to 3 x longer than span - use 16mm rod. Longer buildings to be referred to engineer. End wall cross bracing is not required if Mid Frame used.
- Roof batten with double roof bracing sets require a double roof batten at mid brace locations. Double roof batten can be located to underside of rafter. Strap bracing is to join at the nearest batten adjacent to the Gable Mullion.



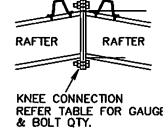
GABLE END ROOF PURLIN DETAIL



WALL GIRT/ ROOF PURLIN LAP DETAIL



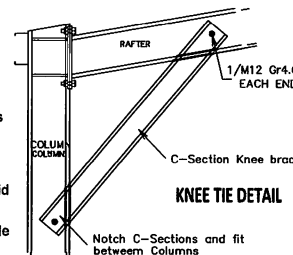
GABLE MULLION TO RAFTER DETAIL



APEX DETAIL



COLUMN BASE DETAIL



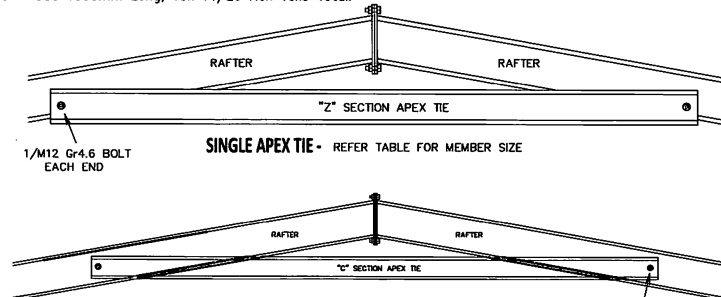
KNEE TIE DETAIL

REFER SPAN TABLES FOR MEMBER SIZES.

SINGLE KNEE TIE

NESTING CHANNEL DETAILS

C100-200 600mm Long, 6x 14/20 Hex Tek's Total.
C250 750mm Long, 8x 14/20 Hex Tek's Total.
C300 - 350 1000mm Long, 10x 14/20 Hex Tek's Total.



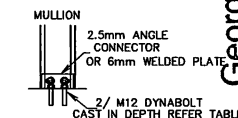
SINGLE APEX TIE - REFER TABLE FOR MEMBER SIZE

REFER SPAN TABLES FOR MEMBER SIZES.

BACK TO BACK KNEE TIE

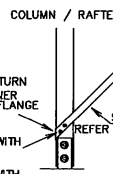
BACK TO BACK APEX TIE - REFER TABLE FOR MEMBER SIZE

1/4 M12 Gr4.6 BOLT EACH END

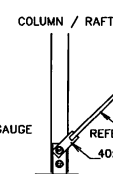


GABLE MULLION TO SLAB DETAIL

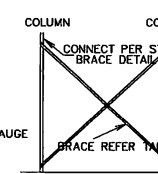
SIZE	CAST IN DEPTH	WEB BOLTS
UP TO C20024	70	2 x TOTAL M12 Gr4.6
C250 +	70	2 x TOTAL M16 Gr4.6



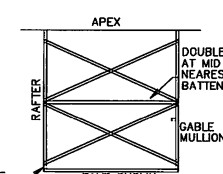
STRAP BRACE



ROD BRACE



WALL BRACING SET



DOUBLE ROOF BRACING SET



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DO NOT SCALE

DRAWING TITLE

GABLE FRAME PORTAL SHEDS

AREA

CONNECTION DETAILS & NOTES
GABLEPORT02

ENGINEERING DOCS.

AMEND DESCRIPTION: FOR CONSTRUCT
DESIGN VERIFICATION: 01.07.2017

BCC DS
RECEIVED
26/08/2026
APPLICATION REF
A007082531

GENERAL NOTES

- If more than 50% of gable wall cladding is removed for openings, doors or windows, End frames shall be replaced with a single Mid frame only. Double or back to back frame not required. Correct spaced End Mullion will be required.
- Refer tables for batten sizes & spacings.
- Maximum of 1800mm Wide Window allowed with no side studs, head or sill and are allowed to be fixed to wall sheet. Windows fixed to wall sheets to have a maximum height no greater than wall batten spacing.

ROLLER DOOR MULLION SELECTION

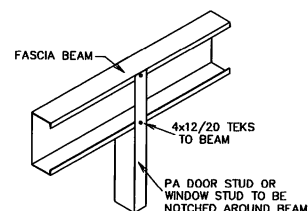
WIND CAT.	3000 HIGH	3600 HIGH	4500 HIGH	5000 HIGH	6000 HIGH	8000 HIGH
N40 / N40D	C10012	C10015	C10019	C15015	C15019	C20019
N50 / N50D	C10012	C10015	C10019	C15015	C15019	C20019
N61 / C50	C10015	C10019	C15012	C15019	C20015	C20019
N61D / C61	C10015	C10019	C15012	C15019	C20015	C20019
C73 / C85	C10019	C15012	C15015	C20015	C20019	C25019

ROLLER DOOR HEADER SELECTION

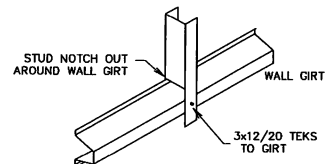
WIND CAT.	2700 WIDE	3000 WIDE	3500 WIDE	4000 WIDE	5000 WIDE	6000 WIDE
N40 / N40D	1.0mm	C10012	C10012	C15012	C15019	C20019
N50 / N50D	1.0mm	C10012	C10012	C15012	C15019	C20019
N61 / C50	1.0mm	C10015	C10019	C15015	C20015	C20019
N61D / C61	1.0mm	C10015	C10019	C15015	C20015	C20019
C73 / C85	C10015	C10019	C15012	C20015	C20024	C25024

GENERAL NOTES

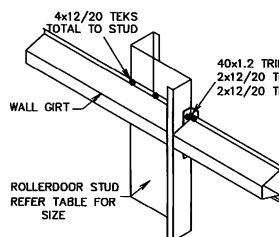
- Optional Head Selection use in plane C100-12 for N40, N40D, N50, N50D. Use C100-15 N61, N61D, C50, C61, C73 & C85. Maximum opening width 6m.
- Roller door mullions can be Z-Section or C-Sections. Fixings acceptable for both.



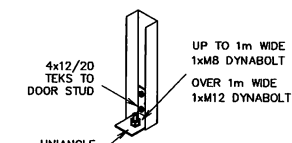
1. PA DOOR / SGD / WINDOW STUD TO FASCIA BEAM DETAIL



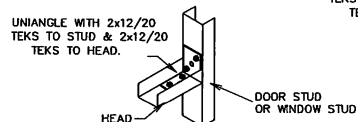
2. WINDOW STUD TO WALL GIRT



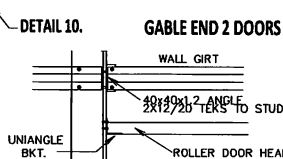
3. WALL GIRT TO ROLLER DOOR STUD



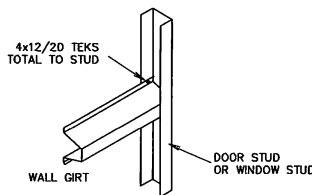
4. PA DOOR / SGD STUD TO SLAB



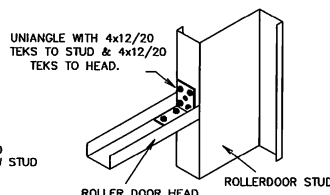
5. DOOR / WINDOW HEAD TO STUD



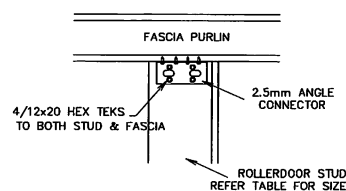
3. ROLLER DOOR STUD TO HEAD DETAIL



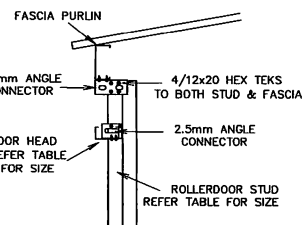
7. DOOR HEAD TO STUD



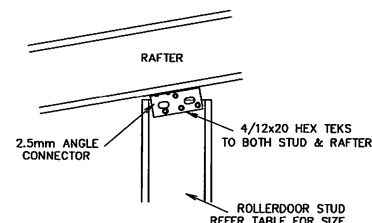
8. ROLLER DOOR STUD TO FASCIA - SUIT 64 TOP HATS



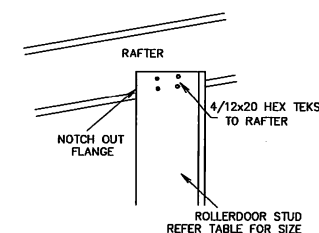
8. ROLLER DOOR STUD TO FASCIA - SUIT 96 & 120 TOP HATS



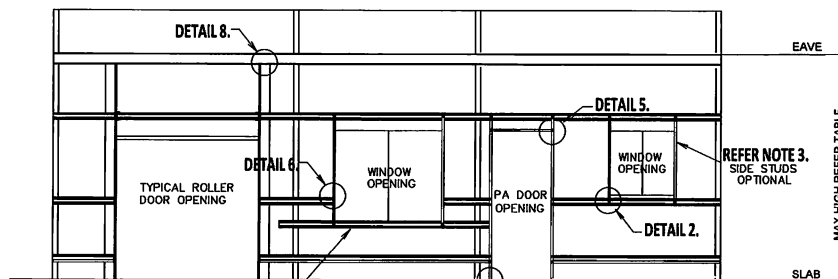
10. ROLLER DOOR STUD TO SLAB



9. ROLLER DOOR STUD TO RAFTER WITH BRACKET OPTION

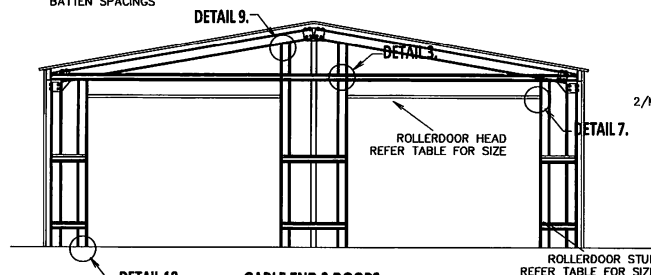


9. ROLLER DOOR STUD TO RAFTER NOTCHED OPTION

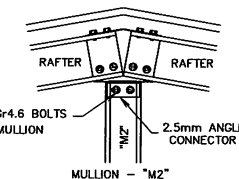


EXTRA BATTEN CAN BE LOCATED TOP OR BOTTOM OF OPENING WHEN WINDOW HEIGHT EXCEEDS BATTEN SPACINGS

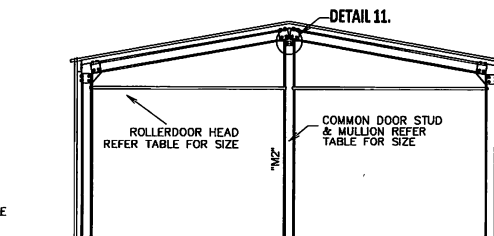
SIDE WALL ELEVATION



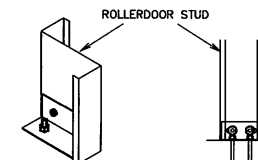
GABLE END 2 DOORS



11. COMMON ROLLER DOOR STUD & MULLION

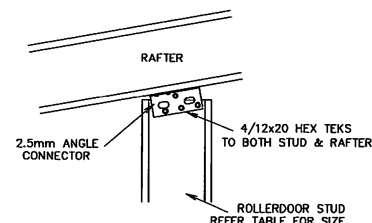


COMMON ROLLER DOOR STUD & MULLION

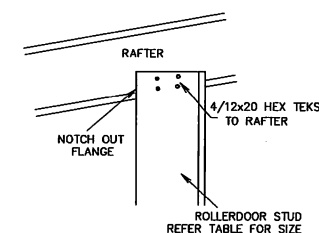


SIZE	CAST IN DEPTH	WEB BOLTS	FLANGE BOLTS
UP TO C20024	70	2 x TOTAL M12 Gr4.6	2 x TOTAL M12 Gr4.6
C250 +	70	2 x TOTAL M16 Gr4.6	2 x TOTAL M16 Gr4.6

10. ROLLER DOOR STUD TO SLAB



9. ROLLER DOOR STUD TO RAFTER WITH BRACKET OPTION



9. ROLLER DOOR STUD TO RAFTER NOTCHED OPTION

NOTES FOR CENTRE END MULLIONS ON FLAT - 'M2'

- NOT SUITABLE FOR USE WITH WINDLOCKED DOOR.
- MAXIMUM SINGLE OPENING SIZE FOR FLAT MULLION ORIENTATION IS 3000w x 2700h.
- FOR LARGER OPENING, END MULLION MUST BE PLACED ON EDGE WITH DOOR MULLIONS EITHER SIDE.

CENTRE END MULLIONS ON FLAT SELECTION

MULLION HEIGHT	MEMBER SCHEDULE						
	N40/N40D	N50/N50D	N61/C50	N61D	C61	C73	C85
3000 HIGH	C15015	C15019	C15019	C15024	C15024	C20019	C20024
4500 HIGH	C15019	C15024	C15024	C20019	C20019	C20024	C25024

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DO NOT SCALE

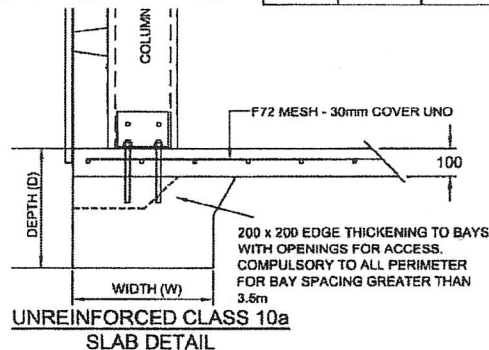
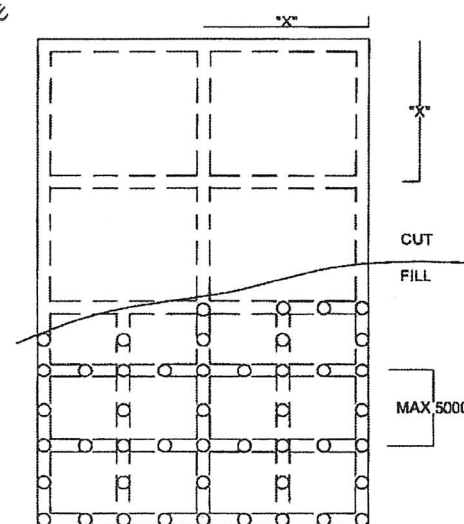
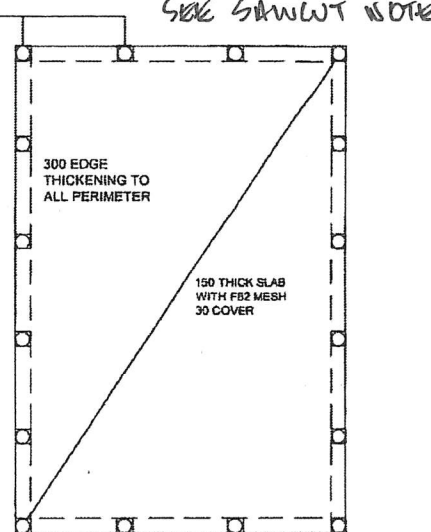
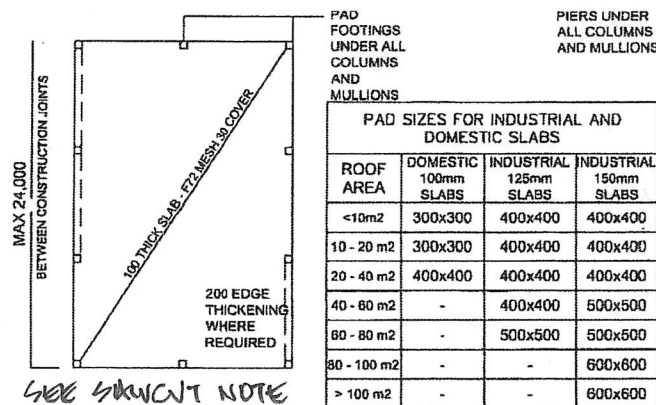
STANDARD SLABS & FOUNDATIONS

12 JUN 2026

George Zuev
RPEQ 7551

DWG. NO. PREC-1010 - 1

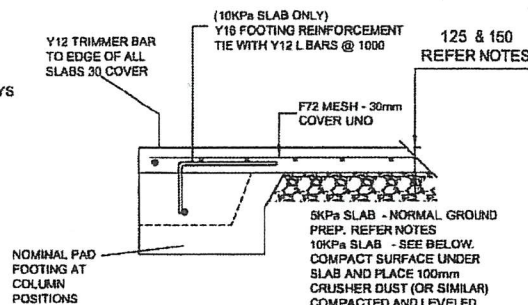
SANUCUT FLOOR SLABS:
AT MAX 6000 CTRS
3W & 30 DEEP
THROUGHOUT



SUITABLE FOR CLASS 10a SHEDS IN FIRM STABLE GROUND. MAX SHRINKAGE - CLASS M & M-D. FOR CLASS H AND H-D SOILS INCREASE SLAB DEPTH TO 110mm AND INSTALL 3000 MASS CONCRETE PIERS UNDER EACH PAD FOOTING TO 1500 BELOW SURFACE. SUITABLE FOR SHED SPANS NOT GREATER THAN 12.0m AND BAY SPACING NOT GREATER THAN 4.5m

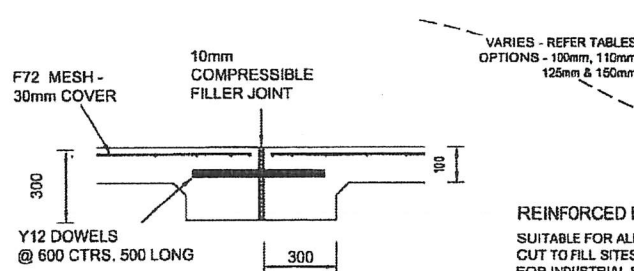
NOTES

- MINIMUM 28 DAY CONCRETE STRENGTH TO BE 20 MPA. MAXIMUM AGGREGATE STONE SIZE - 20mm. DESIGN SLUMP TO BE 80mm ± 10mm.
- EDGE BEAMS TO BE FOUNDED ON NATURAL SOIL OR CONTROLLED COMPACTED FILL.
- SLABS ON CUT/FILL SITES AND APPLICABLE PROBLEM SOILS MAY USE MASS CONCRETE PIERS PLACED UNDER THROUGH FILL 200mm INTO NATURAL SOIL.
- SOIL CONDITIONS ARE ASSUMED TO BE CLASS M OR BETTER FOR STANDARD SLABS UNO. TABLED OPTIONS FOR CLASS H, HD & E ARE ALSO PROVIDED.
- SLAB AND EDGE BEAMS SHALL BE POURED IN ONE CONTINUOUS OPERATION UNO.
- CONCRETE IS TO BE COMPACTED BY VIBRATION OR OTHER MECHANICAL MEANS.
- SAW-CUTTING OF CRACK CONTROL JOINTS SHALL BE CARRIED OUT WITHIN 48 HRS OF THE PLACING OPERATION.
- A SINGLE LAYER OF 200 MICRON PVC SHEETING SHALL BE PLACED UNDER THE SLABS. 50mm CRUSHER DUST IS RECOMMENDED FOR A LEVEL EVEN COMPACTED SURFACE.
- CURING OF SLAB SURFACE AGAINST EXCESSIVE MOISTURE LOSS SHALL BE CARRIED OUT FOR 7 DAYS AFTER PLACING. APPROVED CURING METHODS ONLY.
- SLABS PLACED WIDER THAN 15m SPAN SHALL HAVE F82 MESH AND 150 TH SLAB AS STANDARD.
- ALL SLABS PLACED LONGER THAN 18m (ONE POUR) SHALL HAVE F82 MESH AS STANDARD. MAXIMUM LENGTH OF SLAB BETWEEN CONSTRUCTION JOINTS: 100mm SLAB - 24m, 150mm SLAB - 30m.



SUITABLE FOR CLASS INDUSTRIAL SHEDS IN FIRM STABLE GROUND. MAX SHRINKAGE - CLASS M & M-D. ENSURE PROPER GROUND PREPARATION UNDER SLAB.

5 KPa LIVE LOAD - 125mm SLAB F72 MESH
10 KPa LIVE LOAD - 150mm SLAB F82 MESH



EDGE BEAM AND SLAB SCHEDULE

SITE CLASS	"D"	SLAB MESH	TRENCH MESH	MAX INTERNAL BEAM SPACING "X"	PIERS
A	300	F72	3-8TM	-	-
S	300	F72	3-8TM	-	-
M	300	F72	3-11TM	-	-
M-D	300	F72	3-11TM	8.0m	-
H	400	F72	3-11TM	7.0m	2.5m Ctrs
H-D	400	F82	3-11TM	6.0m	2.5m Ctrs
E	500	F82	3x Y12TM	5.0m	2.5m Ctrs
P	500	F82	3x Y12TM	6.0m	2.5m Ctrs

CLASS E & P SITES SHOULD BE REFERRED TO AN ENGINEER FOR CONFIRMATION. CLASS P DESIGN CAN BE USED FOR CUT/FILL SITES. MINIMUM REINFORCEMENT LAPS:-

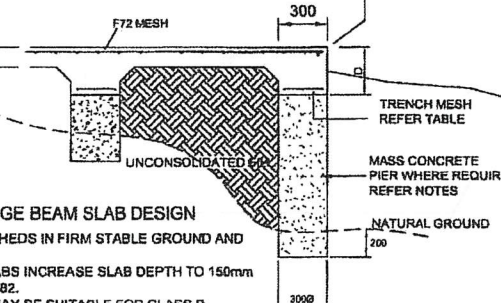
Member	Lap	Member	Lap
Y12	350	F82M MESH	425 end 225 side

ENSURE OUTER BAR OF MESH IS WITHIN 20-30mm OF SLAB EDGE. FOR BOLT SET CLOSE TO SLAB EDGE, A 12mm TRIMMER BAR IS RECOMMENDED.

MAX BEAM SPACING - 5000 RECOMMENDED.

REINFORCED EDGE BEAM SLAB DESIGN

SUITABLE FOR ALL SHEDS IN FIRM STABLE GROUND AND CUT TO FILL SITES. FOR INDUSTRIAL SLABS INCREASE SLAB DEPTH TO 150mm AND MESH SIZE TO F82. THIS DESIGN ALSO MAY BE SUITABLE FOR CLASS P "PROBLEM SITES". REFER TO ENGINEER.

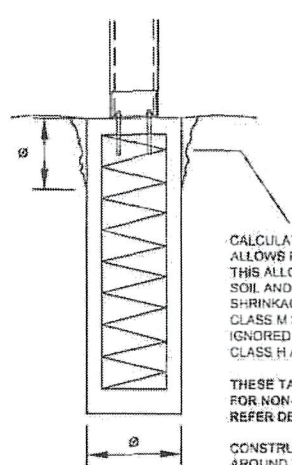


PIER ONLY DETAILS FOR PORTAL FRAME SHEDS

THE TABLE BELOW REPRESENTS THE REQUIREMENTS FOR ENCLOSED SHEDS OF BAY SIZES AND SPANS NOTED. MAXIMUM HEIGHT OF SHEDS FOR THESE FOOTINGS SHALL NOT BE MORE THAN 6M AND NOT MORE THAN 66% OF SPAN.

THE SPAN, BAY AND HEIGHTS NOTED IN THE TABLE REPRESENT MINIMUM REQUIREMENTS

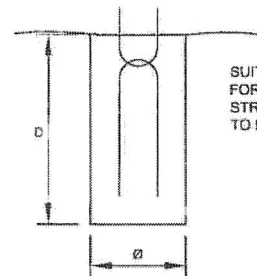
3m BAYS	4m Shed Span		6m Shed Span		7.5m Shed Span		9m Shed Span		12m Shed Span		15m Shed Span		18m Shed Span		21m Shed Span		24m Shed Span	
	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth
N2	300	700	300	700	300	700	300	900	450	900	450	1000	450	1000	450	1000	450	1000
N3	300	700	300	700	300	700	300	900	450	900	450	1200	450	1200	450	1200	450	1200
N4	300	800	300	800	300	900	300	1000	450	1000	450	1500	450	1500	450	1500	450	1500
C1	300	800	300	800	300	900	300	1000	450	1000	450	1500	450	1500	450	1500	450	1500
C2	300	900	300	900	300	1000	300	1200	450	1200	450	1800	450	1800	450	1800	450	1800
3.5m BAYS	4m Shed Span		6m Shed Span		7.5m Shed Span		9m Shed Span		12m Shed Span		15m Shed Span		18m Shed Span		21m Shed Span		24m Shed Span	
	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth
N2	300	700	300	700	300	900	300	900	450	900	450	1000	450	1000	450	1000	600	1200
N3	300	700	300	700	300	900	300	900	450	900	450	1200	450	1200	450	1200	600	1500
N4	300	800	300	800	300	900	300	1000	450	1000	450	1500	450	1500	450	1500	600	1800
C1	300	800	300	800	300	900	300	1000	450	1000	450	1500	450	1500	450	1500	600	1800
C2	300	900	300	900	300	1000	300	1200	450	1200	450	1800	450	1800	450	1800	800	2200
4.5m BAYS	4m Shed Span		6m Shed Span		7.5m Shed Span		9m Shed Span		12m Shed Span		15m Shed Span		18m Shed Span		21m Shed Span		24m Shed Span	
	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth
N2	300	900	300	900	300	900	300	900	450	900	450	1000	450	1000	450	1200	600	1200
N3	300	900	300	900	300	900	300	900	450	900	450	1000	450	1200	450	1200	600	1500
N4	300	900	300	900	300	1000	300	1000	450	1000	450	1500	450	1500	450	1500	600	1800
C1	300	900	300	900	300	1000	300	1000	450	1000	450	1500	450	1500	450	1500	600	1800
C2	300	900	300	1000	300	1200	300	1200	450	1500	450	1800	450	1800	600	2000	600	2000
6m BAYS	4m Shed Span		6m Shed Span		7.5m Shed Span		9m Shed Span		12m Shed Span		15m Shed Span		18m Shed Span		21m Shed Span		24m Shed Span	
	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth	Diam	Depth
N2	300	900	450	900	450	900	450	900	450	1000	450	1200	450	1200	600	1200	600	1200
N3	300	900	450	900	450	900	450	1000	450	1200	600	1000	600	1200	600	1200	600	1500
N4	300	800	450	1000	450	1000	450	1200	450	1500	600	1200	600	1500	600	1500	600	1800
C1	300	900	450	1000	450	1000	450	1200	450	1500	600	1200	600	1500	600	1500	600	1800
C2	300	1000	450	1200	450	1300	450	1500	450	1800	600	1500	600	2000	600	2000	600	2200



CALCULATION OF PIER SKIN RESISTANCE ALLOWS FOR IGNORING THE TOP SECTION. THIS ALLOWS FOR SEASONAL CHANGE OF THE SOIL AND IS DEPENDENT ON THE SOIL'S SHRINKAGE CAPABILITY. CLASS M SOILS - 1 x PIER DIAM SHOULD BE IGNORED. CLASS H AND E SOILS - IGNORE 1.5 x PIER DIAM.

THESE TABLED PIERS WILL NOT BE SUITABLE FOR NON-COHESIVE OR LOOSE FILL SITES. REFER DESIGN TO ENGINEER.

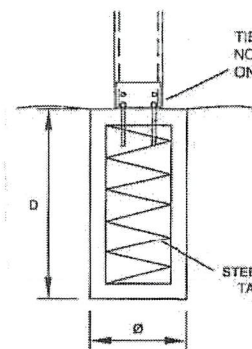
CONSTRUCTING A CONCRETE PATHWAY AROUND THE SHED PIERS AND/OR PIERS AS PART OF THE SLAB WILL INCREASE THE NETT STRENGTH OF A PIER IN GROUND.



TYPE 1 PIERS

SUITABLE ONLY FOR CLASS 10a STRUCTURES UP TO 9M SPAN

REINF	Ø	MAX DEPTH
1 x Y12	300	900
2 x Y12	300	1200
2 x Y16	450	1200



TYPE 2 PIERS

Ø	MAX DEPTH	LIGS	STEEL
450	900	6mm @ 450	4 x Y12
450	1200	6mm @ 300	4 x Y12
450	2000	6mm @ 250	4 x Y12
600	1200	6mm @ 250	6 x Y12
600	1800	10mm @ 300	6 x Y16
600	2600	10mm @ 250	6 x Y16

NOTES

- THESE PIER DETAILS ASSUME A MINIMUM SITE CLASSIFICATION OF SOIL CLASS "M".
- FOR CLASS H & HD SITES, INCREASE DEPTH OF PIER BY ONE PIER DIAMETER. CLASS H PIERS SHALL BE A MINIMUM DEPTH OF 1500mm
- MINIMUM BEARING CAPACITY OF PIER BASE SHALL BE 400KPA.
- FOUNDING PIERS IN FILL SHALL NOT BE PERMITTED.
- CONCRETE SHALL BE A MINIMUM OF N20 WITH A DESIGN SLUMP OF 80 mm ± 20 mm.
- CONCRETE SHALL BE MECHANICALLY COMPACTED OR BY HAND RODDING.
- PIER SHALL BE 1 FT PROUD OF THE GROUND SURFACE 50mm TO 150mm PERMITTED. TOP SHALL BE SLOPED TO ALLOW WATER TO DRAIN AWAY.
- STANDARD RULES FOR A CLASS H SITE ACCORDING TO AS2870 REGARDING SURROUNDING FLORA PLACEMENT SHALL APPLY.
- NON-COHESIVE SOILS SUCH AS SANDS AND LOOSE SILT'S SHALL BE TREATED AS "PROBLEM SITES" AND SHALL NOT BE COVERED BY THIS DRAWING.
- CLASS E AND E-D SITES SHALL ALSO BE ALLOWED USING THESE TABLES WITH THE FOLLOWING PROVISIONS:
 - > PIER DEPTH SHALL BE A MINIMUM OF 1800mm
 - > TYPE 1 PIERS SHALL NOT BE PERMITTED.
 - > INCREASE TABLED DEPTHS BY ONE AND A HALF PIER DIAMETERS
- THE PORTAL SHED DESIGNS FOR THESE PIERS ARE ASSUMED THE FOLLOWING INTERNAL PRESSURE COEFFICIENTS
 - NON-CYCLONIC - +0.2
 - CYCLONIC - +0.7
- MACHINERY SHEDS AND OTHER OPEN SIDED TYPE SHEDS SHALL USE THE PIER DETAILS FOR CYCLONIC CONDITIONS.
- ROOF ONLY BUILDINGS IN C1 CATEGORY SHALL USE N3 CASE FOR PIER SELECTION. C2 WIND CATEGORY SHALL USE N4.
- AWNINGS & END WALL COLUMNS SHALL USE SHED FOOTINGS OF PORTALS CARRYING SIMILAR ROOF AREAS.
- THESE PIER DESIGNS ARE BASED ON A MINIMUM ALLOWABLE SOIL SHEAR STRESS OF 50KPa



THIS DRAWING AND THE INFORMATION THEREON IS THE PROPERTY OF R&F STEEL BUILDINGS AND SHALL NOT BE USED OR REPRODUCED FOR ANY PURPOSE WITHOUT THE WRITTEN CONSENT FROM R&F STEEL BUILDINGS. THESE DRAWINGS ARE SUPPLIED ON THE CONDITION THAT, IN THE EVENT OF ERROR, THIS FIRM'S LIABILITY IS LIMITED ONLY TO THE COST OF AMENDING THESE DRAWINGS.

DO NOT SCALE

STANDARD SLABS & FOUNDATIONS

DWG. NO. RPEL1010-2

12 JUN 2026

George Zuev
RPEQ 7551

DESIGN ALLOWANCES SKIN FRICTION